

Use of Natural Pozzolanic Material as Partial Replacement Ordinary Portland Cement(OPC) in Concrete

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Abstract

Fly ash is a powder material of burned coal from thermal power stations which produces cementitious and pozzolanic material. Commonly use of fly ash in concrete for building construction contributes conducive environmental and also reduces the effect of pollutant in site project. Therefore, this study was conducted to investigate the use of fly ash as partial replacement of cement in concrete as a mean of producing more environmental friendly concrete. The content of fly ash as partial replacement of ordinary portland cement(OPC) is investigated by weight accordingly in range 0%(without fly ash), 10%, 20% and 30% for grade 25. The chemical composition of fly ash was determine using X-ray Fluorescence (XRF). The scanning electron microscope (SEM) was used to determine particle size, shape and texture of fly ash. The mix proportion of concrete was determine using mix design method according to British Standard. The workability of the fresh concrete mixture was evaluated using slump test while compressive strength of cubes concrete was evaluated at 7, 14, 28 and 56 days. A total of 48 cubes concrete with size 100mmx100mmx100mm were made. The optimum compressive strength at all ages of testing was obtained at 10% replacement. Workability decreased with an increased in replacement percentage of fly ash. The results therefore show that fly ash as pozzolanic materials can be used to partially replace ordinary portland cement in production of concrete.

Keywords: Pozzolanic Material, Fly Ash, Compressive Strength and Workability

Introduction

The rapid development of the construction industry in Malaysia requires the use of very high materials, especially concrete building materials. Continuous and uncontrolled use of materials eventually leads to running out of raw material stocks for example non-renewable materials such as aggregates and cement materials. Beside that, Production of Portland cement is energy intensive and accounts for 2.4% of the total global CO₂ emission from industries (Falayi, Okonta, & Ntuli, 2017)

One of the most significant activities stressed by the engineers and scientists related to concrete industry aims at a high percentage of replacement of clinker in cement with secondary raw materials, with the possibility of improvement of cement characteristics and durability of concrete.(Kanojia, Dholia, & Rao, n.d.). Replacement materials are usually available in large quantities and can be used to replace Portland cement in green concrete, which include Fly Ash (Fly Ash is a solid waste product dumped in large quantities by thermal Power Plants in Malaysia).

Fly ash is a fine powder which is an industrial waste that is produced from of burning pulverized coal in electric generation power plants. Fly ash is a pozzolanic material that containing aluminous and siliceous material that forms cement in the presence of water. Fly ash forms a compound similar to cement when existing of lime and water. Fly ash can be classified as class F and class C according to ASTM C618 and grade I and grade II according to I.S. 3812 (Gourav & Venkatarama Reddy, 2014). Gourav et al, (2014) said the classification of Fly Ash were depending on lime content and focused on lime reactivity. These value indicates the reactivity of pozzolanic materials measures by testing a standard cube specimen for compressive strength following the guidelines of I.S. 3812 code.

Choure et al, (2017) reported that using fly ash in concrete mix as replacement of cement, increase its compressive strength. Abu Bakar et al, (2012) stated the compressive strength of bottom ash and cement blended with fly ash showed that the result was lower compared to that of the control concrete. Fly ash can be classified in class C and F. The class F has a low lime (CaO) content and its content of SiO₂ + Fe₂O₃ + Al₂O₃ is greater than 70 (ASTM C 618). Yazici et al, (2012) mentioned concretes in containing fly ash that is used at the same rate instead of cement shows compressive strength of the produced concretes increase as fineness of fly ash increases.

Englehard et al. (1995) mentioned pozzolana is a siliceous and aluminous material that in itself possesses little to no cementitious value. However, in finely divided form and in the presence of moisture, it is able to chemically react with calcium hydroxide (lime) at ordinary temperatures to form compound having cementitious properties. Malhotra, et al, (2000) stated fly ash as one of the potential material in reducing the use of cement in construction as it possesses similar binding ability as of cement while containing little to no hazardous chemical substances, making it a more viable and environmentally friendly alternative. However, the study on chemical composition, compressive strength and workability of concrete containing fly ash is still considerably limited in Malaysia. Therefore, this study aims to investigate fly ash to partially replace cement. The fly ash used in this work was provided by Jimah Energy Ventures Power Plant. Therefore, the proposed admixture will be a perfect component of green concrete and its utilization will be a valuable resource for recycling. Mechanical properties of concrete made with the partial replacement of cement, with Fly Ash concrete were studied in this work.

Materials and Methods

Materials

The cement used was ordinary portland cement in compliance to BS12:1996, while the pozzolanic material used was fly ash with conformance with ASTM C618. The fly ash obtained from Jimah Energy Ventures Power Plant, Negeri Sembilan. Coarse aggregates of nominal size 20mm was used in producing concrete and complied to BS 882:1996. River sand (fine aggregate) was used in this study and complied to BS 882:1996. Clean water was also added in the mixing of concrete. Grade 25 concrete was produced using a water binder ratio of 0.55. Four different mixes were used for this study according British Standard (Department of Environment) mix design method of concrete. An attempt is made by partial replacement of Cement. These four mixes includes a control mix using OPC without of fly ash and other three mixes with fly ash replacement 10%, 20% and 30% by mass of OPC as in the control mix. When compound admixture with Fly Ash is mixed with concrete, the performance of concrete can be improved further due to the synergistic effect and activation of each other.

Experimental Tests

The chemical composition of fly ash was conducted using X-ray Fluorescence (XRF) at Universiti Teknologi Malaysia. The particle size, shape and texture of fly ash was determine using scanning electron microscope (SEM). The mix design of concrete was done according to British method of concrete mix design for grade 25. The slump test was used to evaluate the workability of the mix of concrete. Workability depends on water content, aggregate (shape and size distribution) and cementitious content. The specified size of cube concrete moulds 100mm×100mm×100mm was used. A total of 48 cubes were made. The specimens were made in accordance to BS 1881-116:1983. Demoulding was done after 24 hours and the specimens were placed in a curing tank. The compressive strengths were determined by compressing the concrete cubes at 7, 14, 28 and 56 days using a 2,000kN Matest compression testing machine at Concrete Laboratory, Politeknik Melaka in accordance to BS 1881-116:1983. The specimens were taken out of the curing tank and placed in open air about 2 hours before compress.

Results and Analysis

The chemical compositions of cement and fly ash that used in this study shows in Table 1 while Figure 1 shows the particle size, shape and texture of fly ash. According to ASTM 618-05, class F of fly ash stated as silicon dioxide (SiO_2) + aluminium oxide (Al_2O_3) + iron oxide (Fe_2O_3) is not less than 70% and calcium oxide (CaO) is less than 10%. Result from table 1 shows (SiO_2) + (Al_2O_3) + (Fe_2O_3) is 88% and can be classified as class F.

Table 1
Chemical Composition of OPC and Fly Ash

Component	Ordinary Portland Cement (%)	Pozzolanic Material (Fly Ash Type F)(%)
SiO_2	17–25	60
CaO	60–67	3.36
Al_2O_3	3.0–8.0	21
Fe_2O_3	0.5–6.0	7
MgO	0.1–4.0	1.24
SO_3	1.3–3.0	0.664

Na ₂ O	0.4–1.3	1.5
K ₂ O	0.4–1.3	1.88

Microstructure with Scanning Electron Micrograph (SEM) in Figure 1 shows the SEM of the original fly ash from JIMAH Venture Power plant consist of mostly glassy and spherical particles. Sizes of particles fly ash are 1.0 μ m-7.6 μ m and the fly ash reacts with free lime in the existing of water and converted into calcium silicate hydrate (C-S-H) which is the strongest and durable portion of the concrete. According to Glassy and spherical particles may help to reduce internal friction between aggregates thereby concrete containing fly ash increase consistency and allow mobility for longer pumping distances.

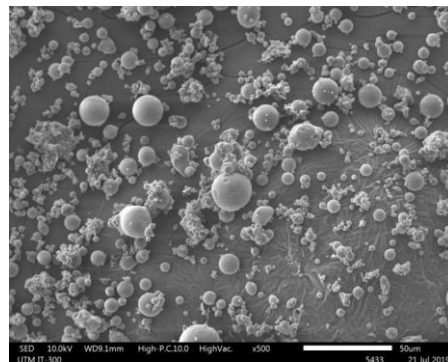


Figure 1: SEM of Fly Ash

The British method of concrete mix design was published by the British Department of the Environment (DOE) and commonly known as “DOE Method”. The details of mix proportions are shown in Table 2. DOE mix design engaged several following stages.

- i. Determination of target strength, f_{ck}
 specified characteristic strength=25 N/mm²
 standard deviation=1.64
 risk factor=8N/mm²
 Target mean strength = specified characteristic strength +(standard deviation x risk factor)

$$= 25 + (1.64 \times 8)$$

$$= 38.12\text{N/mm}^2$$

f_{ck} = Characteristic Compression Strength at 28 days

- ii. Determination of water cement ratio
 Design of water cement ratio=0.55.

- iii. Calculation Of Water Content

W_f = Free water content appropriate to type of fine aggregate

W_c = Free water content appropriate to type of coarse aggregate

Type of slump test 30-60mm

$$\begin{aligned}\text{Water content} &= \frac{2}{3}W_f + \frac{1}{3}W_c \\ &= \frac{2}{3}(180) + \frac{1}{3}(210) \\ &= 190\text{kg/m}^3\end{aligned}$$

Maximum water content required for 20 mm nominal maximum size of aggregate = 190 kg/m³

- iv. Determination of cement content
Mixing water content is 190 kg/m³ of concrete.

$$\begin{aligned}\text{Cement content} &= \frac{\text{Water content}}{\text{Water cement ratio}} \\ &= \frac{190}{0.55} \\ &= 345.45 \text{ kg/m}^3\end{aligned}$$

Maximum cement content required = 345.45 kg/m³

- v. Determination of total aggregate
Water content: 190 kg/m³
Uncrushed aggregate: 20 mm
Specific gravity: 2.6
The wet density = 2375 kg/m³
Weight of Total Aggregate = The wet density - (Weight of Cement + Weight of Free Water)
Weight of Total Aggregate = 2375 - (345.45 + 190)
= 1839.55 kg/m³
- Percentage of fine aggregate = 49 %
Weight of fine aggregate = 1839.55 × 0.49
= 901.38 kg/m³
Weight of coarse aggregate = 1839.55 - 901.38
= 938.17 kg/m³

Table 2

Details of mix proportions of concrete grade 25

Percentage replacement(%)	Weight of Materials					Target Mean Strength (N/mm ²)
	Cement (kg/m ³)	Fly Ash (kg/m ³)	River Sand (kg/m ³)	Coarse Aggregate (kg/m ³)	Water (L)	
0	345.45	0	901.38	938.17	190	38.12
10	310.91	34.5	901.38	938.17	190	38.12
20	276.50	69.0	901.38	938.17	190	38.12
30	242.0	104	901.38	938.17	190	38.12

The details of coarse aggregate sieve analysis shows in Table 3 while Figure 2 shows the semi log graph of coarse aggregate sieve analysis. The coarse aggregate used in experiments were purchased from local market in Melaka. From the results, sample of coarse aggregate in all sizes within the limitation and complied to BS 882:1996

Table 3

Sieve Analysis of Coarse Aggregate

Aggregate size(mm)	Weight Retained(g)	Individual Percent Retained (%)	Cumulative Weight Retained(g)	Cumulative Percent Retained (%)	Calculated Percent Passing (%)
28	0	0	0	0	100
20	2158	78.79	2158.00	78.79	21.21
14	508	18.55	2666.00	97.33	2.67
10	69	2.52	2735.00	99.85	0.15
5	4	0.15	2739.00	100.00	0.00
2.36	0	0	2739.00	100.00	0.00
Pan	0	0	2739.00	100.00	0.00
Total	2739				

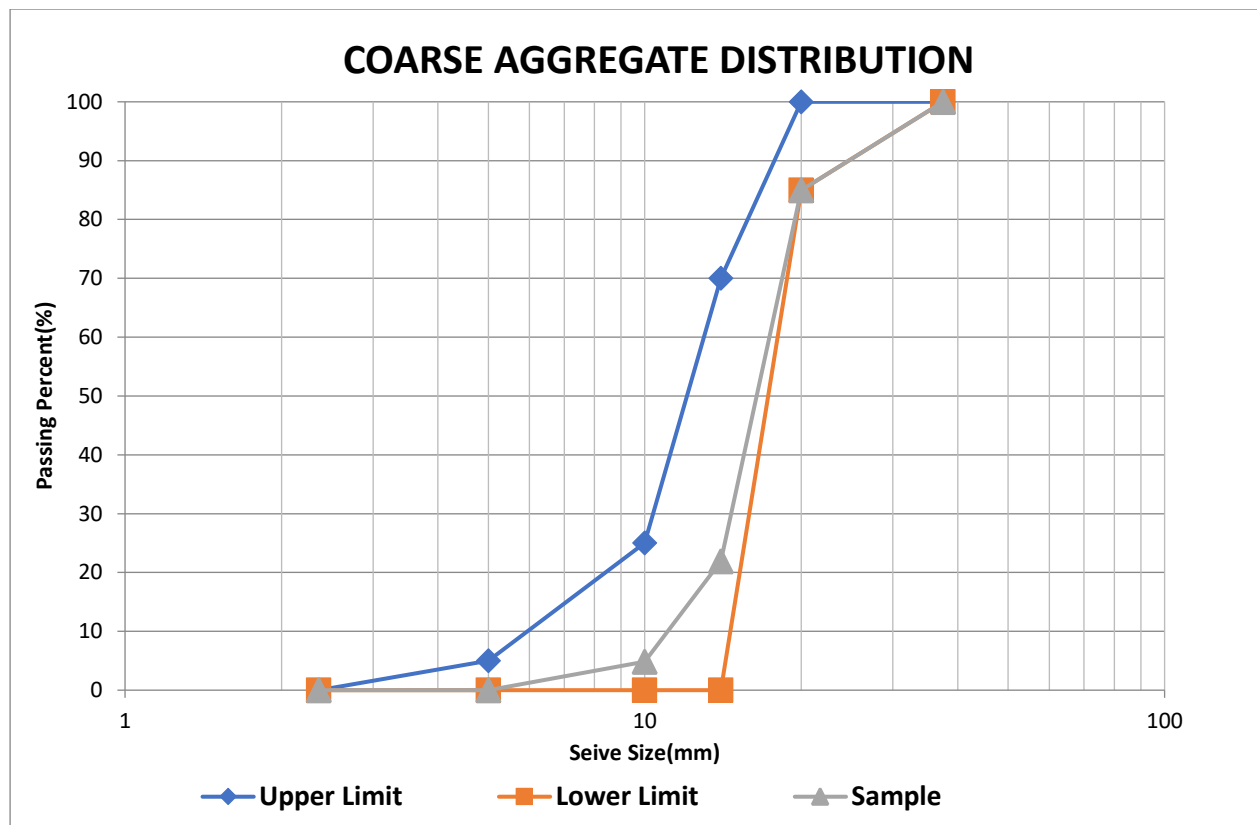


Figure 2: Sieve Analysis of Coarse Aggregate (BS 882:1996)

Table 4 shows the details of fine aggregate sieve analysis while Figure 3 shows the semi log graph of river sand analysis. The river sand used in experiments were purchased from local market in Melaka. From the results, sample of river sand in all sizes is within the limitation and complied to BS 882:1996.

Table 4

Sieve Analysis of River Sand

Aggregate size(mm)	Weight Retained(g)	Individual Percent Retained (%)	Cumulative Weight Retained(g)	Cumulative Percent Retained (%)	Calculated Percent Passing (%)
10	0	0	0	0	100
5	81	7.83	81.00	7.83	92.17
2.36	272	26.31	353.00	34.13	65.86
1.18	324	31.33	677.00	65.47	34.53
0.6	192	18.57	869.00	84.04	15.96
0.3	94	9.09	963.00	93.13	6.87
0.15	60	5.80	1023.00	98.94	1.07
Pan	11	1.06	1034.00	100	0
Total	1034				

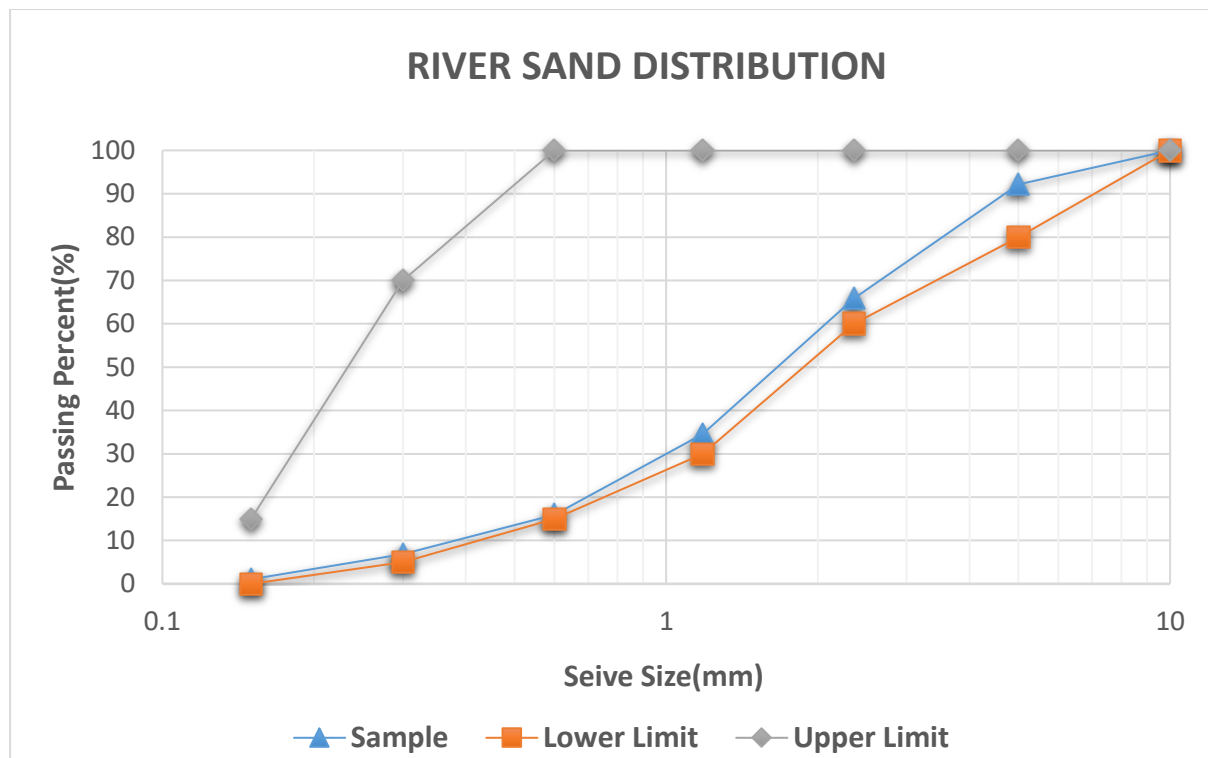


Figure 3: Sieve Analysis of River Sand (BS 882:1996)

Slump Test

The results of workability tests shows in Table 5 while graph of workability of fresh concrete containing a partial of fly ash shows in Figure 4. Generally, it is observed that the percentage of fly ash in mix of concrete increased, the workability of concrete decreased. Therefore, increasing replacement of cement with fly ash will be increased water demand in fresh concrete mix.

Table 5

Workability Test

Workability	Cement Replacement (%)			
	0	10	20	30
Slump (mm)	63	61	60	57

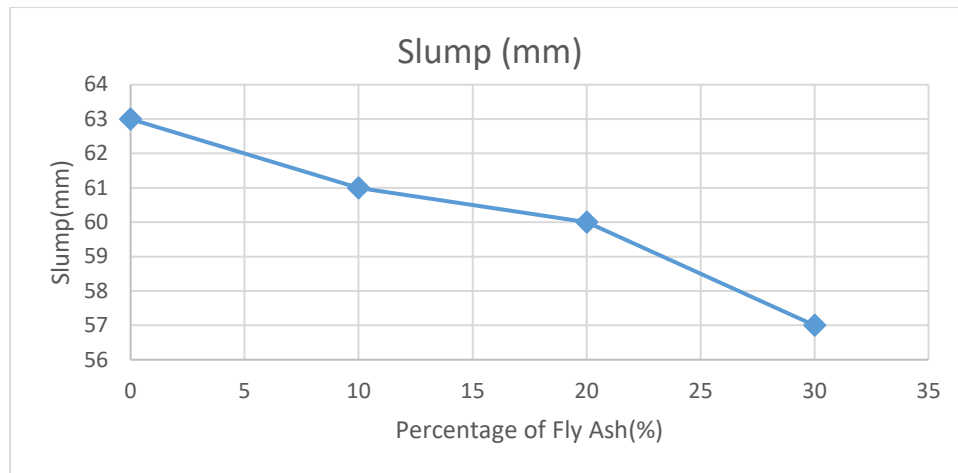


Figure 4: Graph of Slump Test

Compressive Strength

The relationship between compressive strength and difference percentage of fly ash at age 7 days, 14 days, 28 days and 56 days shows in Figure 5 while Table 6 shows the results of compressive strength of concrete with partial replacement of fly ash. The compressive test was conducted to determine the compressive strength of concrete at the age of 7, 14, 28 and 56 days. The compressive strength of concrete also increases with age. The maximum compressive strength (43.57 N/mm^2) was seen at 10% replacement of OPC with fly ash at 56 days and exceed target mean strength (38.12 N/mm^2) but lower than control concrete (40.61 N/mm^2). The increased in compressive strength at 56 days may be due to slow hydration process when increased of fly ash in concrete which is slow reactive pozzolans causes delay the hydration process at 28 days.

Table 6

Compressive Strength (N/mm^2)

Concrete Age (days)	Fly ash replacement percentage (%)			
	0	10	20	30
7	24.15	24.50	27.27	23.20
14	35.33	35.61	30.93	25.03
28	39.89	37.74	34.26	33.54
56	40.61	43.57	37.80	37.61

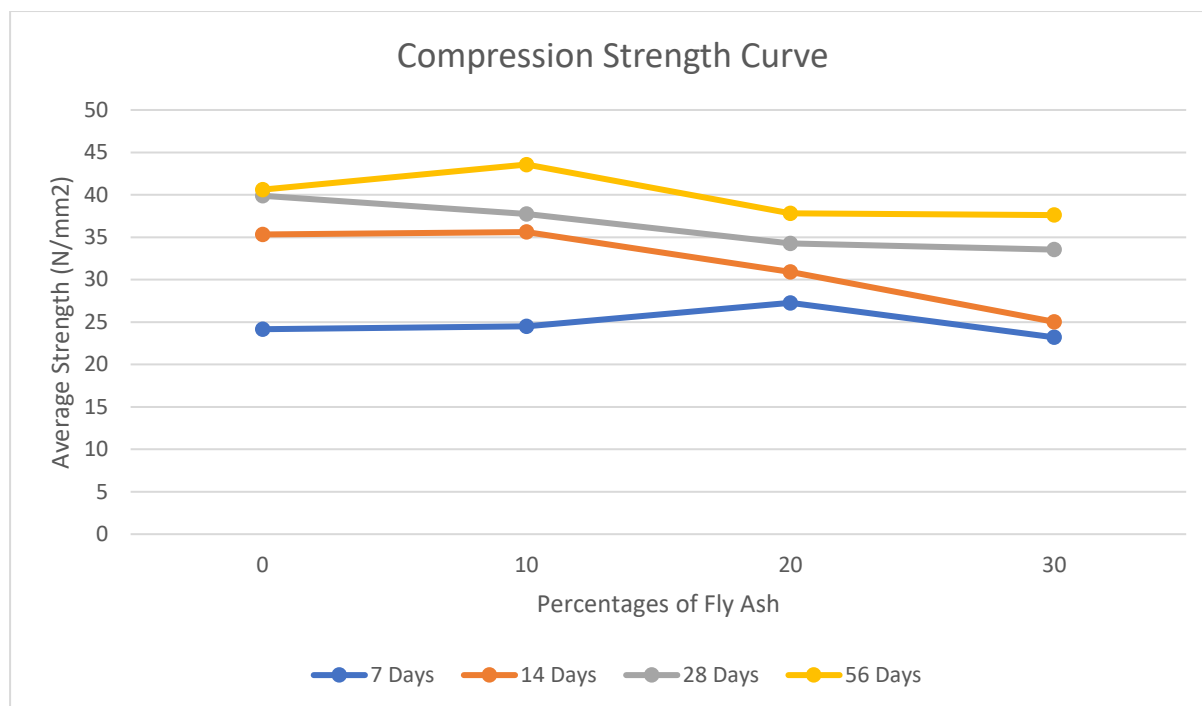


Figure 5: Relationship between compressive strength and difference percentage of fly ash at age 7 days, 14 days, 28 days and 56 days

Conclusion

In this study, fly ash was used as partial replacement of OPC in difference percentage of 0% (control), 10%, 20% and 30%. From the XRF test shows calcium oxide (CaO) is less than 10% can be concluded that fly ash as class F and has a little cementitious properties. Thus, from the SEM, the shape of fly ash is spherical. The spherical shape of fly ash can reduce internal friction thereby increasing the concrete consistency and allow mobility for longer pumping distances. Based on this study, the replacement of cement with fly ash significantly decrease the compressive strength of concrete. From Table 6, it is deduced that fly ash replacement of 10% of the weight of cement achieved the optimum value of compressive strength and considered the best ratio of cement replacement in a concrete mix at 56 days. From the slump test, the increase in fly ash content in concrete decreases the slump height, thus indicating that there is a decrease in the workability of concrete. Therefore, from this study, the partial replacement of cement by fly ash definitely more economical alternative and potential utilize in the field of construction industry with reducing environmental impact to surrounding. However, further study needs to be done on wide range of characteristics of concrete.

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